Micromechanical interpretation of thermo-plastic behaviour of clays

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Abstract. The effect of temperature on mechanical behaviour of clay-based geomaterials is relevant in a number of geotechnical applications (e.g. low enthalpy geothermal systems and energy geostructures, nuclear waste disposal, and heating in rapid shear deformation). Mechanical response of (saturated) clays upon heating is not always intuitive as volume changes may occur due to both thermal expansion of clay constituents and temperature-induced changes of clay microstructure. This paper first revisits the macroscopic thermally-induced mechanical behaviour of saturated clays available in the literature via an advanced thermo-elastoplastic constitutive model and then elucidates the dependence on clay mineralogy of the two key parameters of the model (mechanical hardening and thermal softening respectively) by inspecting differences in clay inter-particle electro-chemical forces occurring in kaolinitic, illitic, and smectitic clays. The micromechanically-based interpretation of constitutive parameters can serve as a guidance for soil parameter selection in the design of energy geostructures.

1 Introduction

The effect of temperature on mechanical behaviour of geomaterials is a relevant problem of growing concern in geoenvironmental engineering. During the last decades, the response of soils to the combined effects of stress and temperature has been studied in depth. Nonetheless, most investigations have been carried out within the context of nuclear waste geological disposal [1, 2] focusing on the study of high-swelling clays subjected to high stress and temperature. The effect of temperature changes has not been extensively investigated in non-swelling or moderately active clays, which are of interest in a number of applications including shallow geothermal plants [3], energy geostructures [4], and shear heating in rapid shear movements [5].

The thermal behaviour of granular materials is governed by the thermal expansion of the grains. When a sandy soil is heated under drained conditions, its volume increases as both grains and water dilate thermo-elastically (proportionally to their thermal expansion coefficients). Due to drained conditions that establish coarse-grained materials, soil water thermal expansion does not contribute to the volume variation of the material itself [4].

Volume changes in (saturated) clays occur due to both thermal expansion of clay mineral and soil water and temperature-induced changes of clay microstructure. The interaction between these two mechanisms is complex and, as a result, mechanical response of clays upon heating is not always intuitive. This includes the volumetric 'collapse' observed in normally consolidated clays upon heating. As a result, a detailed characterisation of the coupled thermo-mechanical behaviour of clays subjected to varying temperature turns out to be fundamental at the design stage.

This paper presents an insight into the experimental evidence available in the literature concerning the macroscopic thermomechanical behaviour of clays. The Advanced Constitutive Model for Environmental Geomechanics with Temperature effect, ACMEG-T [7], developed at EPFL (Switzerland), is deployed to interpret these macroscopic observations. The two key parameters of the constitutive model (mechanical hardening and thermal softening respectively) are interpreted in the light of the different electro-chemical forces assumed to control clay particle interactions in clays of different mineralogy (kaolinite, illite, and smectite).

2 Thermomechanical response of saturated clays

For the case of (saturated) clays, the mechanical response to thermal loading depends on loading history, being thermo-elasto-plastic for Normally Consolidated (NC) clays and thermo-elastic for highly Over-Consolidated (OC) clays (Figure 1) [8 – 12]. The material irreversibly and non-linearly undergoes contraction upon heating in NC conditions, while OC materials experience a reversible volume expansion during heating. An intermediate case is represented by slightly OC clays: the material shows initial dilation and subsequent contraction upon heating, followed by contraction during cooling.

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This macroscopic response of clayey soils to temperature cannot be explained solely in terms of thermal expansion of the solid and pore fluid phases, as is the case of granular materials. Thermal dilations of water and clay minerals are positive (Table 1, [13]) and volumetric response upon heating should be characterised by a volumetric (elastic) expansion if controlled only by thermal dilations of individual phases. However, NC clays undergo volumetric contraction when subjected to an increase in temperature and this volume change is not recovered during subsequent cooling.
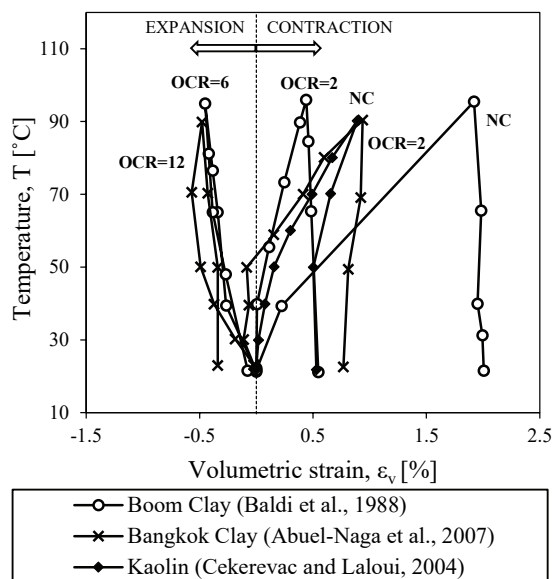

Fig. 1. Thermal volumetric behaviour of saturated clays.

Temperature therefore appears to affect the electro-chemical interactions between clay particles giving rise to a more complex response of the clay-water-electrolyte system. According to [14], the clay particle surface is negatively charged and the particle edge is positively charged when the clay is mixed with 'ordinary' acidic water. Under these conditions, clay microstructure occurs as a house of cards where particles arrange themselves in face-to-face (non-contact) or edge-to-face (contact) configurations (Figure 2). Plastic volumetric deformation occurring upon mechanical loading has been associated with disengagement of edge-to-face contact with particles moving from edge-to-face to face-to-face [14]. For the case of thermally-induced plastic volumetric collapse observed in NC clays, it can therefore be tentatively assumed that an increase in temperature encourages the weakening of the net attractive force at the edge-to-face contact promoting disengagement with consequent plastic deformation [15]. Accordingly, thermally-induced plastic deformation should depend on electro-chemical forces generated between clay-particles, which depend in turn on clay mineralogy. This micro-macro interaction is further explored in the next sections.

Table 1. Volumetric thermal expansion coefficients of soil minerals and water [13].

Material	Volumetric thermal expansion coefficient, β_s [10^{-5} °C^{-1}]
Kaolinite	2.90
Illite	2.50
Chlorite	3.12
Water (β_w)	$13.9 + 0.61T$*

*T being the temperature in °C, considering constant pressure
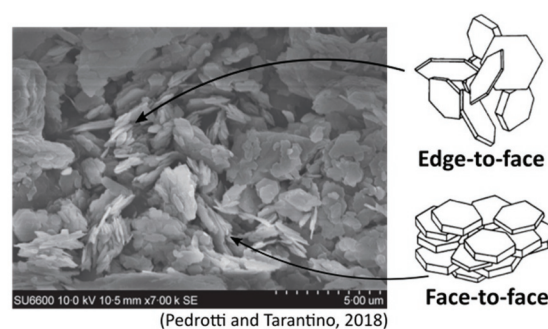

(Pedrotti and Tarantino, 2018)

Fig. 2. SEM image on a Speswhite kaolin clay sample consolidated at 70 kPa [14] from which kaolin clay particles can be identified.

3 Constitutive model

Modelling the complex non-linear and irreversible behaviour of clay, when subjected to combined thermal and mechanical loading, requires advanced constitutive models. During the last thirty years, several elastoplastic models have been developed to take into account the effects of temperature variations on the behaviour of clayey soils [16 – 19]. A Cam-Clay-based constitutive model was formulated at EPFL in Lausanne to unify the different peculiar aspects characterising the thermo-mechanical response of fine-grained soils: the Advanced Constitutive Model for Environmental Geomechanics, with Temperature effects (ACMEG-T) [7].

The model is based on the thermo-elastoplasticity theory and the critical state theory [20]. As the Original Cam-Clay model suffers from some inconsistencies in the prediction of the behaviour of normally consolidated soils, an isotropic yielding mechanism was added to the Original Cam-Clay deviatoric yielding mechanism. According to the theory of multi-mechanism plasticity [21], two thermo-plastic mechanisms are considered: the isotropic mechanism, f_{iso}, and the deviatoric mechanism f_{dev}. This means that there are two different dissipative processes described through two evolution laws, activated by two yield functions, through two dissipative potentials and through two plastic multipliers [22]. The two yield functions define a closed domain in the isotropic effective stress - deviatoric stress - temperature space inside which the behaviour of the material is reversible (Figure 3).

As our purpose is to calibrate temperature-dependent constitutive parameters driving the volumetric thermal

response, we will focus on the isotropic yield function with respect to temperature (horizontal plane in Figure 3).

3.1 Thermo-elastic strain

According to [7], the total deformation is composed of volumetric and deviatoric parts. In the elastic non-isothermal domain, the volumetric deformation is represented by the superposition of a reversible thermal induced strain and a mechanical elastic strain (under isothermal conditions):

$$\varepsilon_v^{Te} = \beta_s \, (T - T_0) + (p' - p'_0)/K \qquad (1)$$

where ε_v^{Te} [-] is the volumetric thermo-elastic strain, β_s [°C^{-1}] is the volumetric thermal dilation coefficient, $(T - T_0)$ [°C] is the temperature variation, $(p' - p'_0)$ [kPa] is the mean effective stress variation (assuming negative stresses when a compression occurs) and K [kPa] the bulk elastic modulus.

3.2 Isotropic Thermoplastic Mechanism

The development of thermo-plastic strain starts when the stress point reaches the yield limit. The yield limit of the isotropic thermoplastic mechanism is expressed by:

$$f_{iso} = p' - p'_c = 0 \qquad (2)$$

where p'_c [kPa] is the preconsolidation pressure at given temperature T, which evolves with the generated volumetric plastic strain ε_v^p [-] :

$$p'_c (T) = p'_{c0} \exp (\beta \, \varepsilon_v^p) \qquad (3)$$

with p'_{c0} [kPa] being the initial preconsolidation pressure at current temperature T and β [-] being the plastic rigidity index, i.e. the slope of the linear function $(\varepsilon_v^p – \log p')$.

Several results from the literature show a decrease in the preconsolidation pressure with increasing temperature. Therefore, the elastic domain under isotropic loading is reduced as temperature increases, which is representative of thermal-softening. The thermal effect on the yield limit is taken into account by considering [23]:

$$p'_{c0} (T) = p'_{c0,T0} [1 - \gamma_T \log(T/T_0)] \qquad (4)$$

where $p'_{c0,T0}$ [kPa] is the initial preconsolidation pressure at temperature T_0 and γ_T [-] is a model parameter.

Finally, the expression of the isotropic thermoplastic yield limit in (2) can be re-written as follows:

$$f_{iso} = p' - p'_{c0,T0} \exp (\beta \, \varepsilon_v^p) \, [1 - \gamma_T \log(T/T_0)] = 0 \qquad (7)$$

Equation (7) depends on two model parameters: β, expressing the evolution of mechanical hardening and γ_T controlling the evolution of thermal softening. The thermal response is actually a combination of these two mechanisms: mechanical hardening and thermal softening [19].
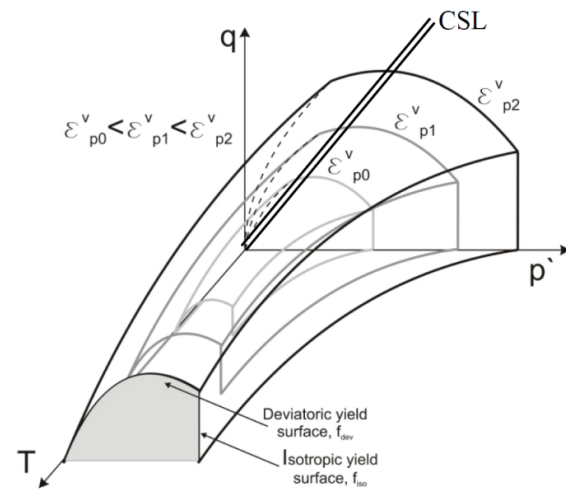

Fig. 3. Yield locus of the ACMEG-T model [4].

This model has been validated on several THM paths but its formulation depends on a number of parameters, which are not easy to identify other than by curve fitting. The challenge addressed in this paper is to provide a micromechanical interpretation to the temperature-related constitutive parameter γ_T and β (Figure 4).

In the mean effective stress-temperature plane shown in Figure 4 the initial isotropic yield function defines an elastic domain (grey space). The value p'_{c0} represents the isotropic preconsolidation pressure at the initial stage. Points A, B and C exemplify three different initial states of the material before applying a drained thermal load at constant mean effective stress. Both points A and C have a current mechanical stress lower than p'_{c0}, thus they are overconsolidated (OC) and their thermal stress path starts in the elastic domain. Point B lies on the yielding locus and represents a normally consolidated material.

Point A represents a highly overconsolidated stress state. This means that when drained heating occurs, the stress state moves from A to A' within the elastic domain without crossing the yield surface. As shown on the (T, ε_v) plane in Figure 4, the material elastically dilates upon heating.

Starting from point B, a positive temperature changes induce thermoplastic strain from B to B', which results in the contraction of the material.

The temperature-stress path of point C starts in the elastic domain but encounters in C'' the isotropic yield surface. The material therefore responds elasto-plastically, dilating elastically until it reaches C'' and then starting contracting when the yield surface is dragged from C'' to C'.

Mechanical loading from B to B'' at constant temperature induces plastic deformation (mechanical strain hardening).
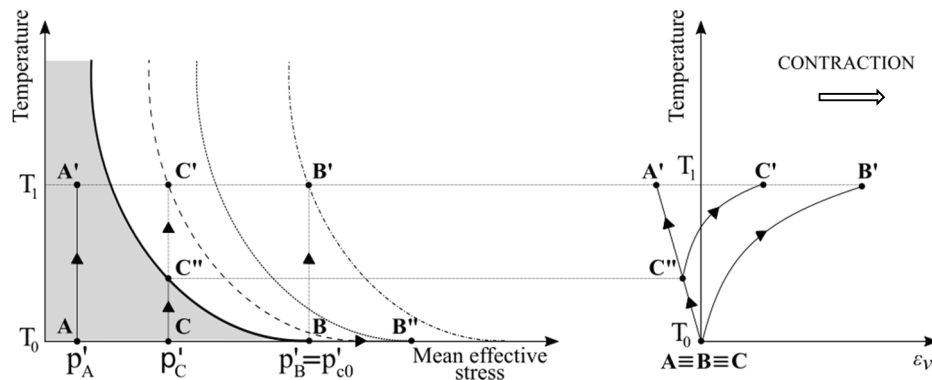
Fig. 4. Yield locus of the ACMEG-T model on (*T, p'*) plane and effects recorded on (*T, ε_v*).

3.3 Calibration of parameters β and γ_T on experimental results

The ACMEG-T model was used to simulate experimental results on five different types of normally consolidated (NC) clays [8 – 12]. According to (1) and (7), the key model parameters to be determined to characterise the thermo-plastic response are γ_T, β and β_s. All the considered tests are done under NC conditions, thus by definition at initial state $p' = p'_c$.

For each experimental dataset, the parameter γ_T in the ACMEG-T model was calibrated by fitting the volumetric strain accumulated during thermal loading at constant isotropic effective stress. The parameter β, corresponding to the plastic rigidity index at constant temperature, was calibrated on isothermal standard mechanical tests provided by the same authors, as proposed by [23]. The volumetric thermal dilation coefficients were chosen according to characteristic values available in the literature [13].

The first example [8] consists of an isotropic drained non-isothermal test on NC remoulded Kaolin clay. The mineralogical content of the tested material was analysed by mean of X-ray diffraction [8]. It revealed the presence of kaolinite (~ 65%) and quartz (~ 35%). The heating-cooling cycle (22°C → 90°C → 22°C) was performed at a confining pressure of 600 kPa. The NC sample contracted during heating with a non-linear volume variation, which was irreversible upon cooling.

The same calibration was performed on a remoulded NC Illite specimen tested in [9] under isotropic condition. After applying an isotropic effective stress equal to 200 kPa, the sample was subjected to a low rate temperature increase until reaching 60°C and subsequently to a controlled cooling down to 5°C.

The model parameter γ_T of the Boom clay was calibrated against an isotropic consolidation test at constant effective stress ($p'_c = 6$ MPa) performed by [10] on natural Belgian Boom Clay. The tested material was a relatively soft, highly plastic, smectitic clay composed by 22% smectite, 29% kaolinite and 19% illite. Even in this case the thermal loading was applied at a very low heating rate under computer controlled zero pore pressure condition, from 20°C to 95°C and then back to 20°.

Natural soft Bangkok clay, obtained from 3.0 to 4.0 m depth, was used in the isotropic and non-isotropic experimental campaign conducted by [11]. The mineralogical composition of the material was analyzed by X-ray diffraction, revealing that this soft Bangkok clay consisted of smectite and illite, ranging from 54 to 71% with kaolinite 28–36% and mica [11]. After imposing an isotropic effective stress of 300 kPa, the specimen was subjected to drained heating and subsequent cooling (22°C → 90°C → 22°C), showing a marked irreversible volumetric strain.

Table 2. Constitutive parameters β, β_s and γ_T evaluated for five different types of clay.

Clay type	Mineralogical composition				Test	β [-]	β_s [10^{-5} °C^{-1}]	γ_T [-]
	Kaolinite [%]	Illite [%]	Smectite [%]	Others [%]				
Kaolin [8]	65	~0	~0	35 (quartz)	ISO	21.0	2.9	0.14
Illite [9]	~0	~96	~0	~4	ISO	22.2	2.5	0.17
Boom Clay [10]	29	19	22	30 (quartz)	ISO	18.0	1.8	0.21
Bangkok Clay [11]	28-36	54-71		1-18 (mica)	ISO	5.5	3.5	0.21
Natural Clay [12]	unknown mineralogical composition				OED	67.0	1.8	0.25
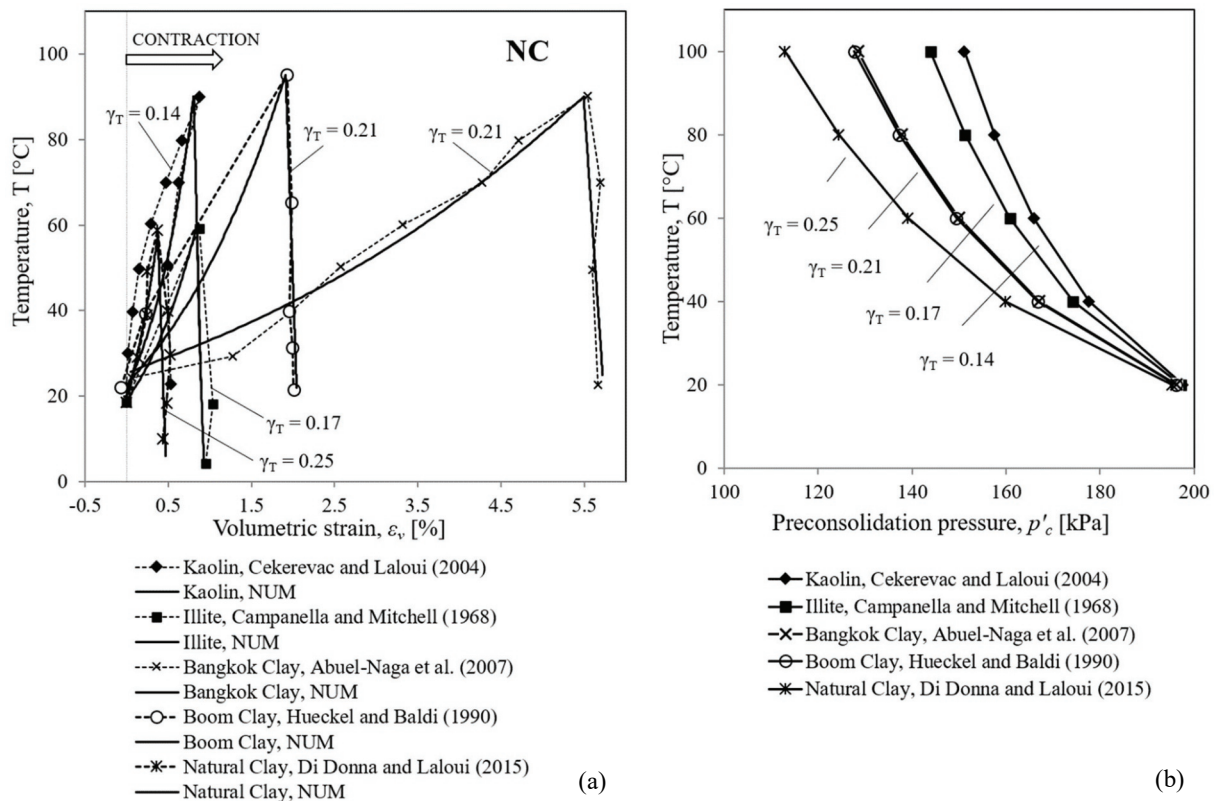
Fig. 5. (a) Calibration of the ACMEG-T parameter γ_T for five different type of clay; **(b)** Shape of the isotropic thermomechanical yield limit for the different values of the parameter γ_T.

Finally, a natural Clay collected in situ near Geneva, in Switzerland was analysed by [12]. According to the material characterisation in [12], the tested clay is poorly plastic (PI<20%) but there is no specific record of the mineralogical content. In this case, the NC specimen underwent a thermal cyclic loading (20°C → 60°C → 5°C) under oedometer condition (σ'_v = 200 kPa).

The calibrated values of β and γ_T are given in Table 2 while the comparison between experimentally observed and predicted volumetric strains for the three aforementioned clays is presented in Figure 5a.

The dependency of the shape of the yield locus on the parameter γ_T is shown in Figure 5b. It appears that γ_T increases as the fraction of active minerals (e.g. smectite). Similarly, the plastic rigidity index β seems to decrease with the fraction of active minerals (compressibility increases).

4 Discussion

The effect of active clay fraction on the parameters β and γ_T is consistent with the micromechanical model discussed in Section 2. Plastic compression at constant temperature is associated with the disengagement of edge-to-face contacts. The weaker is the edge-to-face net attractive force, the higher is the rate of loss of edge-to-face contacts and, hence, the compressibility.

According to [26], alumina, which is exposed at the edges of clay particles, is amphoteric, and it ionizes positively at low pH and negatively at high pH. As a

result, positive diffuse layers can develop at the edges of some clay particles in an acid environment.

The force developing at the edge-to-face contact is the sum of two opposite forces, the van der Waals attraction (activated by the attraction between the positively charged edge and the negatively charged surface) and the Coulombian repulsion between the adjacent negatively charged surfaces as shown in Fig. 6. Since van der Waals forces prevail, the resulting force is attractive. Nonetheless, the intensity of the edge-to-face 'net' attractive force is influenced by the Coulombian (double-layer) repulsion between the negatively charged faces.
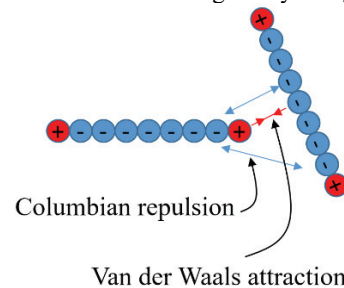
Fig. 6 Sketch of the electrochemical interaction at the edge-to-face contact [15].

The existence of positive edge charges in low pH environment is of greatest importance in kaolinite, lesser importance in illite, and relatively unimportant in smectite

[26]. This can be due to the smaller thickness-to-length ratio of the particle of active clay minerals.

Individual particles of non-swelling clay minerals (such as kaolinite and illite) are much thicker than smectite unit layer platelets and may consist of up to several hundred unit cell layers (see thickness *t* and diameter *a* in Table 3).

Table 3. Particle thickness and particle diameter of some clay minerals [29].

Natural clay	*a* [μm]	*t* [μm]
Kaolinite	0.5 - 4	0.5 - 2
Illite	0.5 – 10	0.003 – 0.1
Smectite (single platelet)	0.05 - 10	0.00095

Smectite clay particles are usually assumed to be a single unit layer platelets in the double layer theory. In reality, this is usually not the case, and the platelets stack into quasi-crystals [27]. For instance, when the adsorbed cation is a divalent cation, a typical quasi-crystal consists of four to seven smectite platelets interspersed by two or three molecular layers of water [28].

For clay minerals with a small thickness-to-diameter ratio such as smectite, the contribution of the termination sites at the edges is small relative to the total charge of the particles. In low pH environment, the positively charged surface-to-total surface area ratio is significantly lower than in illite and kaolinite. The effect of the double layer repulsion between negatively charged faces is predominant in active clay minerals and it influences the intensity of the edge-to-face 'net' attractive force. Therefore, the edge-to-face disengagement turns out to be slightly easier for active minerals thus explaining the decreasing value of *β* with increasing active clay fraction.

In addition, the derivative of the repulsion force F_δ [N] with respect to temperature *T* [°C] according to [15] can be written as:

$$\frac{dF_\delta}{dT} = -\frac{\pi \cdot a^2}{8} \rho^2 \left(\frac{\delta}{\sqrt{a^2 + \delta^2}} - 1\right) \frac{1}{\varepsilon^2} \frac{d\varepsilon}{dT} \qquad (9)$$

where *a* [m] is the clay particle diameter, ρ [C·m^{-2}] is the uniform surface charge distribution, δ [m] is the mid-distance between particle, and ε [C^2·N^{-1}·m^{-2}] is the static dielectric permittivity of the pore fluid. Since $d\varepsilon/dT$ is negative, the derivative dF_δ/dT is positive, i.e. the repulsive force increases with temperature and, hence the net edge-to-face attraction decreases with temperature. This would explain the shape of the yield locus in Fig. 4, i.e. the thermal softening. At the same time, the derivative dF_δ/dT is proportional to the clay particle diameter *a*, i.e. the thermal softening increases with the fraction of larger particles. The particle size *a* has a tendency to increase going from kaolin to illite and from illite to smectite (Table 3). This would partially explain why the values of γ_T found for kaolin are slightly lower than those for illite, which are slightly lower than those for smectite.

5 Conclusions

Mechanical response of (saturated) clays upon heating is not intuitive and includes the volumetric 'collapse' observed in normally consolidated clays. Numerical modelling of thermal effects in clays is relevant in many specific applications. In this paper, an advanced thermo-plastic constitutive model was employed to interpret the macroscopic behaviour of saturated clays.

Some of the thermoelastic-thermoplastic evidences available in literature were reproduced with the aforementioned model. The effect of mineralogy on two key parameters of the model, namely the plastic rigidity index and the thermal softening, was explained in the light of temperature effects affecting electrochemical interactions between clay particles.

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