A Review on Infilled frame Structure with respective of various Interface Materials

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Abstract: During major earthquakes, existing buildings have collapsed or suffered serious damage, resulting in number of losses, severe injuries, and deaths. Based on literature, the influence of this work reviewed the effects of interface with different materials and also to find how infilled frames behave in framed structure. This study's primary goal is to strengthen RC-framed structures and increase the ductility of infilled frames by using interface materials. The research offers a full range and points relevant to ductile parameters for more results in the field of infilled frames using interface materials. In parametric investigation the interface material with interface thickness and the combination of interface material with a particular frame, from that optimum value to be identified. This research benefits researchers, professionals, and specialists the behaviour of various structural systems, as well as innovative mitigation techniques that have been used in the literature to build progressive collapse resistance experimentally.

Keywords: Infilled frame, bare frame, Lateral load, Interface materials & stiffness.

1. Introduction

Unreinforced masonry (URM) walls made of clay/shale/concrete blocks and autoclaved aerated concrete blocks frequently serve as non-structural elements in reinforced concrete (RC) frame structures to offer enclosure and separation roles for buildings. URM infill walls constructed close to the frame components are used in conventional construction techniques. The interaction between the walls and frames is frequently disregarded in favour of converting infill walls into line loads that are applied to the beams in the design. It is well known that infill walls interact with frames in a substantial way, strengthening and stiffening the entire structure[1][2]. Brick units [3]and mortar are combined to form a composite configuration for masonry construction [4],[5] based on the engineering considerations the construction methods are classified into four categories: unreinforced, reinforced, confined, and prestressed. In (RC) buildings[6] the Un-reinforced masonry

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walls are commonly used as in-fills. The most frequent type of construction technology [7] all around the world is the reinforced concrete [8] with mesonry wall [0] [10] Infill walls



Fig. 1. (a) bareframes,(b)fullyinfilled frames,and (c)openground storey frame. [11]

The reinforced concrete [12],[13](RC) buildings the most widely used material is masonry[14]infill walls [15]. The most of the structures[16]and the existing buildings in India [17]-[18][19] the infill is considered as[20],[21]non-structural element [22]-[23]and its impact on structural response is ignored, and the frame and infill do not make proper contact[24].



Fig. 2. Specific articles as published by year in the infilled frame[24]



Fig. 3. Current Construction practice on RC framed structure

Thus we considered if we provide any interface element there is some changes in

the behaviour during earthquake [25],[26]. Hence the element which are used [27] between the Reinforced concrete frame and infill is termed as Interface medium.Earthquake is the most [28] destructive natural disaster to the environment and building structures of all natural disasters.To deal with the risks of seismic damage[29],[30] construction technologies must be advanced and adjusted [31]. It has been discovered that a linear design technique for building construction has failed to account[32]for the inelastic seismic reactions of structures under [33] severe earthquake activities[34], and thus a typical design approach is no longer relevant in terms of long-term risk and benefit implications [35]. In general, frame structures[36] with masonry infill have higher stiffness [37] and strength, and its behaviour changes under cyclic conditions. There are only a few numerical models for the complex topic of the interaction between the frame and the masonry.[38].



Fig. 4. Schematic study of Literature Survey

2. Masonry Materials and Structural System

2.1 Infill Framed Structure

This experimental investigation examines the use of polyethylene foam at the junction[39]of infill masonry walls and surrounding reinforced concrete frames to seismically isolate those structures. This studyto examine how the interaction between the infill and frame changed when polyethylene sheets were used as the interface material. As a result, RC Frame 2's stiffness and strength increased and its behavior became more like that of an infilled frame. Additionally, it was discovered that RC Frame 2's (with PE foam) infill wall damages were less severe than those of RC Frame 1's (with no PE foam).

Studied the Infilled frame structures are[27]] common in buildings around the world as exterior or partition walls for frames for functional reasons. In this study, a single-story frame's bare frame and infilled frame are compared. The infilled frame has a poor stiffness when pneumatic medium with a low air pressure of 2 psi is employed, but a high rigidity when cement mortar is utilized. The effects of various types [38] of masonry infill on how reinforced concrete frames behaveunder lateral stresses is discussed in this work. The properties[40]of the masonry infill were necessary in order to model it. Compression experiments on masonry infill panels and prisms were done to assess the characteristics of brick masonry. Studied that the seismically[41] vulnerable areas of Southern Europe, reinforced concrete frame buildings with masonry infill walls are commonly employed. Infill walls are sometimes overlooked in seismic design and evaluation of existing structures, despite the fact that they can be and often are helpful. Its contribution is particularly important for the seismic fortification of RC frame buildings, where it can be particularly beneficial. Strengthening methods have not been well examined, particularly experimentally, and more research is required. Within the scope of our investigation, we intend to look at possible masonry infill strengthening solutions for both strong and weak frames. Infill walls have a direct effect on the structure's displacement and stiffness response during cyclic stimulation, as indicated in the results. It is well knowledge that frame systems with no infill walls or reinforcing measures have more deformation ability, whereas infilled systems have much higher lateral stiffness and ultimate load. We believe that through a favorable deterioration of structural elements, optimal strengthening procedures can accomplish adequate strength and ductility at the same time. The fact that the extent of damage is greater in the infill walls than in the RC elements is also captured by the strengthening methods.

In comparison to models without any strengthening, the obtained results show that each evaluated strengthening approach somewhat increases the maximum load carrying capacity.

[42]In a variety of computational and experimental research, the impacts of infill walls on the seismic performance of RC structural behavior have been studied. The most frequent cause of failure for such retrofitting frames has been found to be the deboning of the CFRPconcrete adhesive combination. The experimentally observed deboning mode of failure was found to be captured by the cohesive model.

2.2 Un-Reinforced Masonry Structures

Full-scale unreinforced brick masonrywalls[60] with internally retrofitting reinforced concrete layers' in-plane shear strength have been numerically evaluated and is described. The installation of reinforced concrete layers also results in a significant increase in wall capacity and ductility. Studies into a variety of retrofitting [61] and repair strategies have been sparked by research into how susceptible unreinforced masonry (URM) structures are to seismic activity.

Effectivereinforcing techniques [62] are being developed to reduce their sensitivity to earthquakes and, specifically, to prevent the panels from falling. [63] the author examined that the retrofittingis closely tied to the overall building reaction, the seismic behavior of infill walls is a challenging problem. The review and critical comparison of codes and recommendations should be the focus of future research.

2.3 Non-Linear Behavior

Explains the way that brick infills [67] behave structurally and how they interact with frame systems that are susceptible to in-plane loads using the most up-to-date nonlinear modelling techniques currently available. The model combines one or more similar struts with frame components for the beams and columns[68] for the infill panel, is the subject of this research. Modelling of RC or steel frames [69] is expected to include brick infills increasingly frequently as computational rates increase. The macro modelling approach is more often employed in research and practice than the more detailed meso and micro approaches due to

its simplicity and efficiency. Even greater computational capabilities can lead to faster and more stable meso and micro models, however these more refined models are likely to be employed for the study of structural subassemblies rather than entire buildings.

2.4 Seismic Behaviour

The structural behavior of high-rise structures [11],[64] under different earthquake ground motions: effects of infill wall behavior and plan irregularity was investigated in this study. A residential type construction with 15 stories that is largely L shaped has been modelled with all of its parameters. At the mid-section of the structure, there are four lifts/wells, and parking is available on the ground floor.In addition, the effect of infill on irregularity might be calculated. [65]



(a) Infill wall compression and shear damages



(d) OOP infill wall failure



(b) Infill wall shear damage



(e) Infill wall compression and shear damages and moderate frame damage



(c) Infill wall compression damage



(f) Infill wall shear damage and major frame damage

Fig. 5. Effects of various infill wall types on the hysteretic behavior of reinforced concrete frames

[7] The goal of an experiment is to look at reaction parameters for a G+5 story structure. Additionally, research is conducted to determine the potential reduction in column and storey shear caused by the placement of shear walls at a particular location. [66] Old masonry structures and monuments that require modification in seismic regions around the world have been extensively investigated. On the other hand, the codes lack enough information regarding the design of infilled structures and have inconsistent architectural standards that could make structural problems worse. Structures planned and built with or without seismic protections perform similarly if high-strength infills are used.

2.5 Infilled Frame under Lateral Load

[74] The new finite element approach used to investigate brickwork infilled planar frames subjected to lateral loads is described in a companion publication. The effect of the brick infill panel opening on the lowering of the stiffness of the infilled frames has been researched

in the current work.[34] The Infilled frame structures are common in buildings around the world as exterior or partition walls for frames for functional reasons. When cement mortar is used, the infilled frame has a high rigidity, however when pneumatic medium with a low air pressure of 2 psi is used, the infilled frame has a low stiffness.

[45] This paper examines the effect of various masonry infill types on the behaviour of reinforced concrete frames under lateral loads. The results showed that the composite "framed wall" structure had much higher stiffness, damping, and beginning strength than the bare frame structure. The load capacity gap was filled up by masonry infill after the frame assumed control, going from extremely low (0.05 percent) to drifts (0.75 percent).

[75] This work aims to give a simple and clear tool that can reproduce the effects of infills on stiffness and reactions of the frame to earthquake loads.

2.6 Quasi-Static loading

[55] examine the results of an [56] engineered cementitious composites retrofit approach frames made of non-ductile reinforced concrete that are subject to cyclic loads in the plane [57] The mechanical qualities of ECC and masonry pieces were examined to achieve this goal. A simplified analytical approach for calculating the maximum strength of masonry-filled RC frames that have been modified. The results acquired using this method were very similar to those obtained using the experimental method.

[58]Analytical and experimental research revealed that, in comparison to bare frames, brick infilled frames not only increase stiffness and damping properties, but also may cause structural defects. Earthquake effects are created on the frames utilizing a[59] hydraulic actuator with displacement control in a quasi-static way. Varying stiffness, strength, and energy related characteristics are used to compare results from frames with various infill conditions.

3. Interface Medium



Fig. 6. Infill with Interface frame model[43]

[44]Analytical investigations are used to achieve the goal. To study this, we used two

methods to describe the behavior of frame buildings with masonry IF: a linear technique and a Pushovermethod. According to the link axial force, the area where the link force is successfully transferred to the frame by the infill is smaller. This indicates how unused the infill area is. This is due to insufficient strain energy dissipation from the interface to the infill, or poor stress transmission. To strengthen the frame in the case of a seismic event, this needs to be fixed. [45] The results show that adding infill significantly increases the initial fracture load and ultimate load. An infilled frame has a stiffness that is roughly greater by 2.0 than an exposed, bare frame. The ferro-cement mesh bands placed along the bed joints of the reinforced concrete brick masonry infilled frame beat the other infills tested in terms of strength and stiffness.

The effects of several interface materials [46] on a naked frame and infilled frames were studied. The findings of this work can be used to guide future research into the behavior of these interface materials when subjected to seismic loading. [27] The theoretical method was used to verify the experimental results of the first cycle. In the seismic region, infilled frame constructions will be the preferable alternative, as bare frame structures have a lower ultimate load than other infilled frames, which may cause the frames to collapse prematurely during significant earthquake shaking.

Additionally, the analysis aims to assess the dynamic properties and elastic behavior [47] of the stiffness of the infilled frame with a variety of partial interfaces under monotonic in plane loads. This paper uses the ABAQUS program to project an observational analysis of the two bay a three-story frame filled with reinforced concrete. The criticism focuses on the twenty-lateral five's stiffness, partial interface frames are not. The findings of this analysis can aid future research on materials for the partial interface. Compared to bare frames, infilled frames exhibit greater lateral rigidity. In comparison to a frame with a lead or cork interface, one with a cement mortar interface is stronger. Infilled frames filled with regular cement mortar have the highest rigidity among the frames under consideration.

Benefits of Interface Medium

- There are several benefits to using interface medium in infilled frame structures. These include:
- Improved Seismic Performance: One of the main benefits of using interface medium is that it can help to improve the seismic performance of the structure. This is because it allows the infill material to move independently of the frame during an earthquake, reducing the risk of damage and collapse.
- Reduced Shear Forces: Another benefit of interface medium is that it can help to reduce the shear forces that are transferred between the infill material and the frame. This can help to improve the overall stability of the structure.
- Increased Durability: By reducing the risk of damage to both the infill and the frame, interface medium can help to increase the overall durability of the structure.

Applications

Interface medium has been used in a variety of different types of infilled frame structures, including low-rise buildings, high-rise buildings, and bridges. It has also been used in retrofitting existing structures to improve their performance.

Masonry panels' effects on a building's performance:

- Building structures' with more rigidity [48]
- Natural vibrational amplitudes and frequencies
- The capability to dissipate energy [49]
- The failure mechanism and brittle behaviour.

Common infill masonry panels have the potential to significantly alter the overall behaviour

of a structure by drawing forces to areas that were not intended to support them, resulting in unexpected collapse mechanisms.

3.1 Experimental works carried out in Infilled frames



(d) Spalling of concrete at beam end (e) Exposed steel reinforcement (f) Damage of column foot

Fig. 7. Failure characteristics of the specimen [50]

In the above Fig. 7, the various failure characteristics of the frame structure in two bay two storied.

In addition, two infill wall typesone with and one without RC core columns at its two ends —were researched. There were two different kinds of connections, which had a layer of masonry mortar between the frame and infill and flexible connections, which had a gap of 30mm wide and filled with polystyrene plate[5].

The fly ash hollow blocks' holes, into which fine aggregate concrete was poured, served as t he foundation for the core column. Vertical steel bars were then inserted into the holes.



Fig. 8. Details of Experimental set up and Instrumentation[51]

[52]addressed the setup and testing of a novel masonry infill construction methodthe frame with both a flexible and predictable loading along the axis of the surface and perpendicular to the surface. The infill downgrade benefits the building by minimising post-earthquake damage, but it also makes it easier to predict the seismic reaction of the structure given the well-known uncertainty related to standard infill walls. The infill contribution, which is drastically reduced by the proposed technique, might then be factored into a few changes.



Fig. 9. Different Arrangements of Infill in RC Frames [53]



Fig. 10. Idealized load-displacement relation for RC frames with masonry fill [51]

In the above fig. 10, the author stated that the idealised load-displacement curve suggested in the study is based on a small study with numerous unknowns, so it is important to use the findings with caution for the future study.

The lateral drift levels identified in the present study can be used to construct a hypothetical load-displacement connection by taking into consideration the particular points corresponding to the beginning of major occurrences.[54]The critique aims to examine the dynamic properties of the infilled frame with pneumatic interface, such as stiffness and energy dissipation, as well as the elastic behaviour of the system.

Based on the significant damage events (performance levels) noticed during the experimental study, an idealised load-displacement relationship was presented. This might be used as a standard when creating infilled frames with the same performance requirements. It is crucial to keep in mind that the failure mechanism of RC frame systems must be carefully considered while building RC frames loaded with weak brickwork. [55] Using the commercial finite element package SAP 2000,two-dimensional numerical investigations were conducted.By altering thicknesses of 6, 8, 10, 14, and 20 mm, the impact of various cement mortar, cork, and foam thicknesses as interface materials was examined.

[56]The development of a reasonable approach for corresponding diagonal struts can aid in the design of reinforced concrete infilled frames. Different aspects impacting the behaviour of reinforced concrete infilled frames under cyclic loading have been investigated using numerical parametric analyses. The comparable diagonal strut was designed using a rational method that can aid in the design of reinforced concrete infilled frames. [57]examined the one-storey, fully infilled frames with various types of masonry and subjected to cyclic loading was conducted in order to increase using experimental data on the dynamic loading of infill walls frame structures as a guide, the above modelling approach was created.

3.2 Failure modes

The majority of the damage is primarily caused by the material's low

tensile strength. The characteristics of the seismic event have a direct impact on structural damage. The materials utilised, bonding ability, axial normal stress from the top and other superstructures on the wall, opening placement, and seismic loading can all have an impact on how the constructions fail. These complete failure modes do not, however, always happen during an event. Assume that there is some way to stop the cracks from forming and spreading. In that situation, it will be possible to reduce structural damage and, more importantly, during a major disaster occurs.



Fig. 11. Various failure mode in a masonry structure [12]

Infill walls may potentially have negative effects on the mode of failure of RC frame structures, including the concentration of forces in frame elements as a result of the connection with the infill wall and torsional effectscaused by awry arrangements in plane and soft-story effects caused by awry elevations.



Fig. 12. Analogy between the corner crushing (CC) and diagonal compression (DC) modes in the strut model for in-filled frames[58]

Steel Frames:

[59],[60]The goal of this work is to report the findings of an experimental investigation of steel frames with gravity load designs. Masonry walls come in a variety of geometrical layouts for the frame and masonry walls, as well as different types of materials [61]. The hysteresis model of infilled frames is constructed and described on the findings. All of the model's parameters have physical meaning and are calibrated using experimental data.

[50]In comparison to the moderately earthquake-damaged structure, the results

demonstrated a 15.3% increase in the intact structure's capacity to withstand a global collapse when it was reinforced with an enclosed steel jacket. In the event of significant damage, the steel jacket reinforcement's ability to survive a complete collapse after restoration reached 98.5% of the original construction.

Energy dissipation:

[62]In general, the seismic reinforcement of existing buildings involves both the strengthening of existing structures as well as the restoration of structures following natural disasters like fires and earthquakes. By implementing reinforcement measures, the building's original condition can be successfully restored, its seismic performance can be enhanced, and its seismic response can be decreased. Repairing damaged components, enhancing the building's stiffness distribution, strengthening the current structure, installing damping and vibration isolation devices, and lowering the buildings self-weight are the main components of concrete engineering's shear wall reinforcement. [55]According to a review of the literature, little research has been done on the effect of the elastic characteristics of the interface material on the behavior of structural infilled frames. This research is even the more important now, given the new trend of using insulating material at the interface instead of traditional stiff elastic cement mortar. Numerical and experimental investigations are used to achieve the goal.

Pushover analysis:

[63]As can be seen different stories with infill walls show more stiffness and high strength during the initial stages of displacement when compared to the existing structures without masonry walls. [64]As can be seen, the initial strength of the infilled frame is roughly two times higher than that of the frames without infill walls before wall problems begin to appear.[65]It is noteworthy that the pushover curve of the frames weakens due to the equivalent diagonal strut buckling.

4. Conclusion

In conclusion, interface medium is a promising new technology that can help to improve the performance of infilled frame structures. The use of an interface medium between the infill and the frame can improve the ductility response of infilled framed structures. By providing a buffer zone between the infill material and the frame, it can reduce the risk of damage and collapse during seismic events and other types of structural loading. Additionally, because the use of infill material is not covered by the current rule and is regarded as a non-structural component, the choice of infill material is at random. Masonry infill wall panels improve the building's overall ductility, strength, and stiffness. More importantly, they significantly lessen the demand for ductility and deformation on RC frame members. While a panel loaded in its plane is extremely inflexible and collapses after only a minor displacement when built of typical building materials, the frame is rather flexible and fails under excessive load via relative rotation of the members, or parts of them. The objectives are clear of performance-based design is to build the structure in such a way that it meets all of the performance requirements under various ground motions while also resisting seismic forces.

5. References:

- [1] Q. Huang, Z. Guo, J. S. Kuang, Q. Su, and G. Cai, *Eng. Struct.*, vol. **123**, no. 7, pp. 341–353, (2016), doi: 10.1016/j.engstruct.2016.05.024.
- [2] Q. Huang, Z. Guo, and J. S. Kuang, *Eng. Struct.*, vol. **123**, pp. 341–353, (2016), doi: 10.1016/j.engstruct.2016.05.024.
- [3] O. Iuorio, J. A. Dauda, and P. B. Lourenço, *Constr. Build. Mater.*, vol. **269**, p. 121358, 2021, doi: 10.1016/j.conbuildmat.(2020).121358.
- [4] I. S. Misir, O. Ozcelik, S. C. Girgin, and S. Kahraman, *Struct. Eng. Mech.*, vol. 44, no. 6, pp. 763–774, (2012), doi: 10.12989/sem.2012.44.6.763.
- [5] Q. Peng, X. Zhou, and C. Yang, *oil Dyn. Earthq. Eng.*, vol. **108**, no. December 2017, pp. 96–110, (2018), doi: 10.1016/j.soildyn.2018.02.009.
- [6] R. S. Ju, H. J. Lee, C. C. Chen, and C. C. Tao, *J. Constr. Steel Res.*, vol. **71**, pp. 119–128, (2012), doi: 10.1016/j.jcsr.2011.10.004.
- [7] Y. Li, S. Yin, and H. Lv, *Structures*, vol. **33**, no. May, pp. 2226–2237, 2021, doi: 10.1016/j.istruc.2021.05.089.
- [8] I. Caliò and B. Pantò, *Comput. Struct.*, vol. **143**, pp. 91–107, 2014, doi: 10.1016/j.compstruc.2014.07.008.
- [9] S. K. Vyas and U. B. Choubey, *Int. J. Eng. Res. Adv. Technol.*, vol. **3**, no. 11, pp. 15–31, (2017), doi: 10.7324/ijerat.2017.3154.
- [10] T. Sevil, M. Baran, T. Bilir, and E. Canbay, *Constr. Build. Mater.*, vol. 25, no. 2, pp. 892–899, (2011), doi: 10.1016/j.conbuildmat.2010.06.096.
- [11] Z. Baig and D. N. Kakade, vol. 3, no. 1, pp. 45–56, 2017.
- [12] S. Yadav *et al.*, *Prog. Disaster Sci.*, vol. **10**, (2021), doi: 10.1016/j.pdisas.2021.100149.
- [13] T. Kalman Šipoš, H. Rodrigues, and M. Grubišić, *Eng. Struct.*, vol. **171**, no. April, pp. 961–981, (2018), doi: 10.1016/j.engstruct.2018.02.072.
- [14] A. K. Sinha, Int. J. Civ. Eng. Technol., vol. 8, no. 2, pp. 537–546, 2017.
- [15] M. Teguh, *Procedia Eng.*, vol. **171**, pp. 191–200, (2017), doi: 10.1016/j.proeng.2017.01.326.
- [16] F. Anić, D. Penava, L. Abrahamczyk, and V. Sarhosis, vol. 18, no. 5. Springer Netherlands, (2020). doi: 10.1007/s10518-019-00771-5.
- [17] C. Zavala, M. Diaz, E. Nora, and F. Terreros, no. December, (2020).
- [18] F. Mazza, M. Mazza, and A. Vulcano, *Soil Dyn. Earthq. Eng.*, vol. **109**, no. October 2017, pp. 209–221, (2018), doi: 10.1016/j.soildyn.2018.02.025.
- [19] A. H. Karimi, M. S. Karimi, A. Kheyroddin, and A. A. Shahkarami, *Structures*, vol. 8, pp. 144–153,(2016), doi: 10.1016/j.istruc.2016.09.012.
- [20] M. Bikçe, E. Emsen, M. M. Erdem, and O. F. Bayrak, *Eng. Struct.*, vol. 245, no. August, p. 112920, (2021), doi: 10.1016/j.engstruct.2021.112920.
- [21] F. J. Pallarés, A. Davia, W. M. Hassan, and L. Pallarés, *Eng. Struct.*, vol. 235, no. January, (2021), doi: 10.1016/j.engstruct.2021.112031.
- [22] G. Zewdie Tsige, *Am. J. Civ. Eng.*, vol. **6**, no. 1, p. 24, (2018), doi: 10.11648/j.ajce.20180601.15.
- [23] M. Surana, M. Pisode, Y. Singh, and D. H. Lang, *Eng. Struct.*, vol. 175, no. November 2017, pp. 861–878, (2018), doi: 10.1016/j.engstruct.2018.08.078.
- [24] Y. Lu, H. Hao, P. G. Carydis, and H. Mouzakis, *Eng. Struct.*, vol. 23, no. 5, pp. 537–547, (2001), doi: 10.1016/S0141-0296(00)00058-4.
- [25] W. Jin, C. Zhai, J. Kong, W. Liu, and M. Zhang, *Eng. Struct.*, vol. 246, no. August, p. 113079, 2021, doi: 10.1016/j.engstruct.2021.113079.
- [26] S. M. W. Hammoudah, M. T. A. Chaudhary, and A. S. Essawy, *Eng. Struct.*, vol. 171, no. June, pp. 779–793, (2018), doi: 10.1016/j.engstruct.2018.06.035.
- [27] V. Thirumurugan, M. Sekar, and T. P. Ganesan, Natl. Conf. Recent Adv. Sustain. Civ. Eng. Dep. Civ. Eng. Velammal Eng. Coll. Velammal Nagar, Chennai, no.

April, (2015).

- [28] H. Baghi, A. Oliveira, J. Valença, E. Cavaco, L. Neves, and E. Júlio, *Eng. Struct.*, vol. **171**, no. June, pp. 476–487, 2018, doi: 10.1016/j.engstruct.(2018).06.001.
- [29] V. Thirumurugan, T. P. Ganesan, K. S. Satyanarayanan, N. Parthasarathi, and M. Prakash, *Mater. Today Proc.*, vol. 40, no. April, pp. S45–S51, (2020), doi: 10.1016/j.matpr.2020.03.491.
- [30] R. Marques and P. B. Lourenço, *Eng. Struct.*, vol. **64**, pp. 52–67, 2014, doi: 10.1016/j.engstruct.(2014).01.014.
- [31] A. K. Hashmi and A. Madan, "Curr. Sci., vol. 94, no. 1, pp. 61–73, (2008).
- [32] K. Sassun, T. J. Sullivan, P. Morandi, and D. Cardone, *Bull. New Zeal. Soc. Earthq. Eng.*, vol. 49, no. 1, pp. 98–115, (2016), doi: 10.5459/bnzsee.49.1.98-115.
- [33] H. Alwashali, D. Sen, K. Jin, and M. Maeda, *Eng. Struct.*, vol. **189**, no. February, pp. 11–24, (2019), doi: 10.1016/j.engstruct.2019.03.020.
- [34] F. J. Crisafulli, A. J. Carr, and R. Park, *12th World Conf. Earthq. Eng.*, no. January, pp. 1–8, (2000).
- [35] Q. Xu, T. Zhang, J. Chen, J. Li, and C. Li, *Structures*, vol. **32**, no. 2, pp. 355–379, 2021, doi: 10.1016/j.istruc.2021.03.007.
- [36] Maidiawati and Y. Sanada, *Earthq. Eng. Struct. Dyn.*, vol. **46**, no. 2, pp. 221–241, 2017, doi: 10.1002/eqe.2787.
- [37] . C. H., *Int. J. Res. Eng. Technol.*, vol. **04**, no. 04, pp. 640–647, (2015), doi: 10.15623/ijret.2015.0404110.
- [38] V. Sigmund, I. Guljaš, and J. Zovkić, *Teh. Vjesn.*, vol. **21**, no. 2, pp. 389–399, (2014), [Online]. Available: https://hrcak.srce.hr/120393
- [39] Z. Umar, S. A. Ali Shah, T. Bibi, K. Shahzada, and A. Ahmad, *J. Build. Eng.*, vol. 40, no. September 2020, p. 102736, 2021, doi: 10.1016/j.jobe.2021.102736.
- [40] D. V. Prasada Rao and G. Sulochana, "Int. J. Civ. Eng. Technol., vol. 7, no. 1, pp. 180–187, (2016).
- [41] M. Grubišić and V. Sigmund, *Bauhaus Summer Sch. 2012, Course Model Valid.* Simulation, Bauhaus-Universität Weimar, Weimar, Ger.,(2012).
- [42] A. S. Mohamed, R. E. Saher, A. S. Ayman, and A. D. Essam, *Civ. Eng. J.*, vol. 3, no. 4, pp. 267–286, (2017).
- [43] S. Muthu Kumar and K. S. Satyanarayanan, *Mater. Today Proc.*, vol. **5**, no. 2, pp. 8986–8995, 2018, doi: 10.1016/j.matpr.2017.12.343.
- [44] S. Muthukumar, K. S. Satyanarayanan, and K. Senthil, *Struct. Eng. Mech.*, vol. 64, no. 5, pp. 543–555, (2017), doi: 10.12989/sem.2017.64.5.543.
- [45] J. M, P. M H, K. S. B. Narayan, and K. Venkataramana, *IOSR J. Mech. Civ. Eng.*, vol. 11, no. 2, pp. 51–57, 2014, doi: 10.9790/1684-11255157.
- [46] S. Muthukumar, V. Thirumurugan, and K. S. Satyanarayanan, *Disaster Adv.*, vol. **9**, no. 5, pp. 13–17,(2016).
- [47] K. V. Maruthish, K. S. Satyanarayanan, S. Pradeep, and S. Muthukumar, *Mater. Today Proc.*, vol. 43, pp. 3256–3260, 2021, doi: 10.1016/j.matpr.(2021).01.935.
- [48] Maidiawati, J. Tanjung, Y. Hayatfi, and H. Medriosa, *MATEC Web Conf.*, vol. **258**, no. January, p. 05009, (2019), doi: 10.1051/matecconf/201925805009.
- [49] C. K. Kawan, "J. Sci. Eng., vol. **3**, no. March, pp. 7–20, 2015, doi: 10.3126/jsce.v3i0.22383.
- [50] C. Xu, C. Guo, Q. Xu, and Z. Yang, *Structures*, vol. **33**, no. May, pp. 3433–3442, 2021, doi: 10.1016/j.istruc.2021.06.032.
- [51] S. H. Basha and H. B. Kaushik, *Eng. Struct.*, vol. **111**, pp. 233–245, (2016), doi: 10.1016/j.engstruct.2015.12.034.
- [52] M. Preti and V. Bolis, *Eng. Struct.*, vol. **132**, pp. 597–608, (2017), doi: 10.1016/j.engstruct.2016.11.053.

- [53] H. B. Kaushik, D. C. Rai, and S. K. Jain, *Earthq. Spectra*, vol. 22, no. 4, pp. 961– 983, (2006), doi: 10.1193/1.2360907.
- [54] S. Pradeep, P. B. Dinakar, and K. S. Satyanarayanan, *Mater. Today Proc.*, vol. 14, pp. 257–263, 2019, doi: 10.1016/j.matpr.(2019).04.145.
- [55] K. Senthil, S. Muthukumar, S. Rupali, and K. S. Satyanarayanan, *IOP Conf. Ser. Mater. Sci. Eng.*, vol. **330**, no. 1, 2018, doi: 10.1088/1757-899X/330/1/012113.
- [56] R. P. Dhakal, B. H. H. Peng, R. C. Fenwick, A. J. Carr, and D. K. Bull, ACI Struct. J., vol. 111, no. 4, pp. 777–788, (2014), doi: 10.14359/51686732.
- [57] L. Cavaleri and F. Di Trapani, *Soil Dyn. Earthq. Eng.*, vol. **65**, pp. 224–242, 2014, doi: 10.1016/j.soildyn.2014.06.016.
- [58] M. K. S, "crvihoefcrvif," vol. 18, no. 1, pp. 133–149, (2017).
- [59] K. M. Mosalam, R. N. White, and P. Gergely, *J. Struct. Eng.*, vol. 123, no. 11, pp. 1462–4169, 1997, doi: 10.1061/(asce)0733-9445(1997)123:11(1462).
- [60] Z. Li, M. He, X. Wang, and M. Li, J. Constr. Steel Res., vol. 140, pp. 62–73, (2018), doi: 10.1016/j.jcsr.2017.10.012.
- [61] Y. Liu and P. Manesh, *Eng. Struct.*, vol. **52**, pp. 331–339, (2013), doi: 10.1016/j.engstruct.2013.02.038.
- [62] H. Jiang, X. Liu, and J. Mao, *Eng. Struct.*, vol. **91**, pp. 70–84, (2015), doi: 10.1016/j.engstruct.2015.02.008.
- [63] A. Jalaeefar and A. Zargar, *Structures*, vol. **28**, no. May, pp. 766–773, (2020), doi: 10.1016/j.istruc.2020.09.029.
- [64] A. N. Kolekar, Y. P. Pawar, D. C. P. Pise, D. D. Mohite, and S. S. Kadam, *Int. J. Eng. Res. Appl.*, vol. 07, no. 05, pp. 45–52, (2017), doi: 10.9790/9622-0705024552.
- [65] D. Das and C. V. R. Murty, *Indian Concr. J.*, vol. 78, no. 8, pp. 31–38, 2004.